FULCRUM
CONSULTING SERVICES

PLANNERS * ENGINEERS * ARCHITECTS

June 16, 2021

County of Kleberg
Regional Public Safety Training and Command Center

ADDENDUM No. 1

Gentlemen and Madams,

1. The Non-Mandatory Prebid conference of the project has been changed from the original time on June 21, 2021 at 1:00 P.M. to June 24, 2021 at 6:00 P.M. The Non-Mandatory Prebid conference will be held at the JK Northway Expo Center located at 501 E. Escondido Road Kingsville, Texas.

2. The bid date of the project has been changed from the original date on June 28, 2021 at 3:00 P.M. to July 12, 2021 at 1:30 P.M. Sealed bids for this project will be received at the Annex Building located at 1500 E. King Avenue, Kingsville Texas until 1:30 P.M.; Bids will be publicly opened and read aloud at this time.

3. Attach please find additional plan sheets.
   - G001- DRAWING INDEX AND CONVENTIONS
   - C-1.0 COVER SHEET
   - REVISED STRUCTURAL
     - S1.0 GENERAL NOTES
     - S1.1 GENERAL NOTES
     - S1.2 FOUNDATION PLAN
     - S1.3 FRAMING PLAN
     - S1.4 ENDWALL
     - SD1 FOUNDATION DETAILS
     - SD2 FOUNDATION DETAILS

Thank you,

Approved for Electronic Transfer by:
George E. Lazaro, P.E.
Fulcrum Consulting Services

Office: (956) 797-3411 P.O. Box 530540 Harlingen, Texas 78553 Fax: (956) 797-3400

ABBREVIATIONS

[illegible]

MATERIAL LEGEND

THERE IS GENERAL LEGEND. ALL SYMBOLS MAY NOT APPLY.

- EARTH (UNDISTURBED)
- EARTH (COMPACTED)
- POROUS FILL (STONE OR GRAVEL, ETC.)
- ROCK
- STRUCTURAL CONCRETE
- BRICK (COMMON OR FACE)
- CONCRETE MASONRY UNITS (CMU)
- GLAZED STRUCTURAL FACING TILE (GSFT)
- CUT STONE
- METAL (LARGE SCALE)
- METAL (SMALL SCALE)
- ACOUSTICAL CEILING TILE (SECTION / LIT)
- PLASTER, SAND CEMENT, GROUT
- GYPSUM DRYWALL
- GLASS
- RIGID INSULATION
- BATT INSULATION
- ROUGH WOOD (CONTINUOUS)
- ROUGH WOOD (NON-CONTINUOUS)
- FINISH WOOD
- PLYWOOD

SYMBOL LEGEND

THERE IS GENERAL LEGEND. ALL SYMBOLS MAY NOT APPLY.

- SECTION
- ENLARGED PLAN/SECTION
- CONSTRUCTION NOTE
- DEMOLITION NOTE
- DOOR NUMBER
- EQUIPMENT NOTE
- WINDOW TYPE
- INTERIOR ELEVATION
- REVISION NUMBER

ROOM NAME

- ROOM NAME & NUMBER
- PARTITION TYPE

LOCATION MAP

INDEX OF DRAWINGS

GENERAL

G000	COVER SHEET
G001	DRAWING INDEX AND CONVENTIONS
G002	GENERAL NOTES
G003	ACCESSIBILITY NOTES & DIAGRAMS

CIVIL

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C-3.1	PROPOSED SITE PLAN
C-3.2	SITE DEMOLITION PLAN
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C-5.3	SWPPP NOTES
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C-7.0	DRIVE AND PARKING LOT PLAN
C-8.0	AREA DETAILS
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STRUCTURAL

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ARCHITECTURAL

A100	OVERALL FLOOR PLAN
A101	ENLARGED FLOOR PLAN
A102	ENLARGED FLOOR PLAN
A103	ENLARGED PLAN LAYOUTS
A104	OVERALL SQUARE FOOTAGE FLOOR PLAN
A200	REFLECTED CEILING PLAN
A201	REFLECTED CEILING PLAN
A202	REFLECTED CEILING PLAN
A300	ROOF PLAN
A400	EXTERIOR ELEVATIONS
A500	BUILDING SECTIONS
A600	WALL SECTIONS
A601	WALL SECTIONS
A700	INTERIOR ELEVATIONS
A800	SCHEDULES
A900	TYPICAL MILLWORK DETAILS
A1000	SECTION AND WALL DETAILS

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P-3.0	WATER PLUMBING PLAN
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P-4.1	PLUMBING DETAILS

BUILDING CODE SUMMARY	
BUILDING CODES	
INTERNATIONAL BUILDING CODE, 2015 EDITION	
INTERNATIONAL ENERGY CONSERVATIONS CODE, 2015 EDITION	
INTERNATIONAL FIRE CODE	
TEXAS ACCESSIBILITY STANDARDS (TAS), 2012 EDITION	
TYPE OF CONSTRUCTION	
INFORMATION FROM CHAPTER 6 TYPES OF CONSTRUCTION	
CONSTRUCTION CLASSIFICATION: TYPE II-A	
PROTECTED NON-COMBUSTIBLE	
SECTION 602.2 AND TABLE 601	
USE AND OCCUPANCY CLASSIFICATION	
REGIONAL PUBLIC SAFETY TRAINING	
AND COMMAND CENTER	
OCCUPANCY CLASSIFICATION:	
GROUP A-3 – ASSEMBLY (IBC 303.4)	
GROUP B – BUSINESS (IBC 304.1)	
SPRINKLER SYSTEM PROVIDED: YES	
OCCUPANCY SEPARATION: 1 HOUR (TABLE 508.4)	
GENERAL BUILDING HEIGHTS AND AREAS	
ALLOWABLE HEIGHT AND BUILDING AREAS	
(TABLE 504.3, 504.4, 506.2)	
BUILDING HEIGHT: ALLOWED: 85'-0"	ACTUAL: 27'-4"
GROUP A-3 OCCUPANCY	
NUMBER OF STORIES ALLOWED:	4 ACTUAL: 1
GROUP B OCCUPANCY	
NUMBER OF STORIES ALLOWED:	4 ACTUAL: 1
BUILDING AREA PER	
STORY (SQFT) ALLOWED: 62,000	ACTUAL: 16,754
MEANS OF EGRESS	
INFORMATION FROM CHAPTER 10 – MEANS OF EGRESS	
OCCUPANT LOAD (PER TABLE 1004.5)	
A-3 OCCUPANCY AREA	3,812 NET SF / 15 NET SF PER OCCUPANT = 254
B OCCUPANCY AREA	8,747 NET SF / 150 GROSS PER OCCUPANT = 58
TOTAL OCCUPANCY LOAD: 254 + 58 = 312	
MINIMUM NUMBER OF EXITS FOR OCCUPANT LOAD	REQUIRED: 2 PROVIDED: 8
(PER TABLE 1006.3.2)	

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CONSULTANT:

EOC KLEBERG COUNTY

CLIENT:
KLEBERG COUNTY

KINGSVILLE, TEXAS

REGISTERED ARCHITECT
REYNOLDS LEE
STATE OF TEXAS

MAY 16, 2021

This drawing is an instrument of service and the ARCHITECT will retain ownership and property interest therein. The OWNER may make and retain copies for information and reference. However, this drawing shall not be reused without written authorization by the ARCHITECT.

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DO NOT scale drawings.

DRAWING INDEX AND CONVENTIONS

G001

REGIONAL PUBLIC SAFETY TRAINING AND COMMAND CENTER

KLEBERG COUNTY, TEXAS

CIVIL PLANS

KINGSVILLE, TX
SCALE: N.T.S.

FINAL PLAN DATA

FINAL PLAN DATA: _____
CONTRACTORS NAME: _____
CONTRACTORS ADDRESS: _____
LETTING DATE: _____
DATE WORK BEGAN: _____
DATE WORK COMPLETED: _____
DATE OF ACCEPTANCE: _____

CHANGE ORDERS AND SUPPLEMENTAL AGREEMENTS

ALL CONSTRUCTION WORK WAS PERFORMED IN ACCORDANCE WITH THE PLANS SPECIFICATIONS AND CONTRACT. ALL PURPOSED CONSTRUCTION WAS COMPLETED UNLESS OTHERWISE NOTED.

_____ DATE
GEORGE LAZARO, P.E.
FULCRUM CONSULTING SERVICES

LOCAL ENTITIES:

CITY OF KINGSVILLE CONCURRENCE: _____ DATE: _____
NAME _____ TITLE _____

KLEBERG COUNTY CONCURRENCE: _____ DATE: _____

GENERAL NOTES:

IF CULTURE MATERIALS ARE ENCOUNTERED DURING CONSTRUCTION WORK SHALL CEASE IN THE IMMEDIATE AREA AND THE CONTRACTOR SHALL NOTIFY THE OWNER OR ENGINEER. THE OWNER OR ENGINEER SHALL CONTACT THE SECRETARY OF THE INTERIOR, (202-343-4104) IN ACCORDANCE WITH 36 CFR800. THE STATE HISTORIC PRESERVATION OFFICER (512-463-6069) SHALL ALSO BE NOTIFIED.

ALL SOILS ARE UNCLASSIFIED SOILS INCLUDING ROCK OR OTHER MATERIAL. NO SPECIAL PAYMENT WILL BE MADE TO THE CONTRACTOR FOR UNFORESEEN UNCLASSIFIED SOILS ENCOUNTERED. NO SPECIAL PROVISIONS ARE BEING MADE FOR UNCLASSIFIED SOILS.

SHEET INDEX

CIVIL

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C-9.0	GENERAL DETAILS

MECHANICAL

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ELECTRICAL

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E-2.0	ELECTRICAL NOTES
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PLUMBING

P-1.0	PLUMBING NOTES
P-1.1	PLUMBING NOTES
P-1.2	PLUMBING NOTES
P-2.0	SCHEDULE
P-3.0	WATER PLUMBING PLAN
P-3.1	SANITARY SEWER PLAN
P-4.0	PLUMBING DETAILS
P-4.1	PLUMBING DETAILS

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CONSULTANT:

REGIONAL PUBLIC SAFETY TRAINING AND COMMAND CENTER

CLIENT:
KLEBERG COUNTY

KINGSVILLE, TEXAS

ISSUED FOR REVIEW 02/24/2021

P.E.
STATE OF TEXAS
GEORGE LAZARO
5-18-21
F-12369

This drawing is an instrument of service and the ENGINEER will retain ownership and property interest therein. The OWNER may make and retain copies for information and reference; however, this drawing shall not be reused without written authorization by the ENGINEER.

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DO NOT scale drawings.

COVER SHEET

C - 1.0

GENERAL NOTES

GENERAL

1. THESE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE INDICATED, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION. SUCH MEASURES SHALL INCLUDE, BUT NOT BE LIMITED TO, BRACING, SHORING FOR EARTH BANKS, FORMS, SCAFFOLDING, PLANKING, SAFETY NETS, SUMPS AND BRACING FOR CRANES AND GIN POLES, ETC. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND HE SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT OR THE ENGINEER DO NOT INCLUDE INSPECTION OF THE ABOVE AND BELOW ITEMS.
2. ALL CONSTRUCTION AND QUALITY OF MATERIALS SHALL COMPLY WITH THE GOVERNING BUILDING CODES AND REGULATIONS.
3. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS AND CONDITIONS AT THE JOB SITE BEFORE COMMENCEMENT OF WORK AND SHALL IMMEDIATELY REPORT ANY DISCREPANCIES OR OMISSIONS TO THE ARCHITECT IN WRITING. ANY OMISSION OR CONFLICT BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
4. IN CASE OF CONFLICT, NOTES AND DETAILS ON THE BALANCE OF THE DRAWINGS TAKE PRECEDENCE OVER STANDARD NOTES AND TYPICAL DETAILS.
5. WHERE CONSTRUCTION DETAILS ARE NOT SPECIFICALLY SHOWN OR NOTED FOR ANY PART OF THE WORK, SUCH DETAILS SHALL BE CONSTRUCTED IN ACCORDANCE WITH DETAILS SHOWN FOR SIMILAR CONDITIONS AND MATERIALS. WHERE SUFFICIENTLY SIMILAR WORK IS NOT SHOWN, THE ENGINEER SHALL BE CONSULTED FOR CLARIFICATION.
6. EACH SUBCONTRACTOR IS CONSIDERED AN EXPERT IN HIS RESPECTIVE FIELD AND SHALL, PRIOR TO THE SUBMISSION OF BID OR PERFORMANCE OF WORK, NOTIFY THE GENERAL CONTRACTOR OR OWNER OF ANY WORK CALLED OUT ON THE DRAWINGS IN HIS TRADE THAT CANNOT BE GUARANTEED.
7. THE CONTRACTOR SHALL COORDINATE ALL MECHANICAL AND ELECTRICAL EQUIPMENT, AS TO WEIGHTS AND EXACT LOCATIONS WITH STRUCTURAL SUPPORTS. IN THE EVENT THAT THE PURCHASED EQUIPMENT DEVIATES IN WEIGHT AND LOCATION FROM THOSE INDICATED ON THE PLANS, THE ARCHITECT MUST BE NOTIFIED AND APPROVAL OBTAINED PRIOR TO INSTALLATION.
8. THIS STRUCTURE IS DESIGNED AS A STABLE UNIT AFTER ALL COMPONENTS ARE IN PLACE. THE CONTRACTOR SHALL BE RESPONSIBLE TO PROVIDE TEMPORARY BRACING AS REQUIRED TO INSURE THE VERTICAL AND LATERAL STABILITY OF THE ENTIRE STRUCTURE, OR ANY PORTION THEREOF, DURING CONSTRUCTION.
9. NEITHER THE OWNER NOR THE ARCHITECT NOR THE ENGINEER WILL ENFORCE SAFETY MEASURES OR REGULATIONS. THE CONTRACTOR SHALL DESIGN, CONSTRUCT AND MAINTAIN ALL SAFETY DEVICES, INCLUDING SHORING AND BRACING, AND SHALL BE SOLELY RESPONSIBLE FOR CONFORMING TO ALL LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS, LAWS AND REGULATIONS.
10. TRADE NAMES AND MANUFACTURERS REFERRED TO ARE FOR QUALITY STANDARDS ONLY. SUBSTITUTIONS WILL BE PERMITTED AS APPROVED BY THE ENGINEER.
11. ANY OPTIONS OR APPROVED SUBSTITUTIONS ARE FOR CONTRACTORS CONVENIENCE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CHANGES, ADDITIONAL COSTS AND COORDINATION WITH ALL ITEMS THAT THE SUBSTITUTIONS MAY IMPACT.
12. THE ARCHITECT IS TO BE NOTIFIED IN WRITING WHEN CONSTRUCTION AT THE SITE BEGINS.
13. ANY QUESTIONS RELATED TO INTERPRETATION OF INTENT OF THESE DRAWINGS SHALL BE REFERRED TO THE ENGINEER.
14. THE CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO LOCATE AND PROTECT ANY EXISTING UNDERGROUND OR CONCEALED CONDUIT, PLUMBING OR OTHER UTILITIES PRIOR TO BEGINNING ANY WORK.
15. PIPES, DUCTS, SLEEVES, CHASES, ETC. SHALL NOT BE PLACED IN BEAMS OR WALL UNLESS SPECIFICALLY SHOWN OR NOTED. NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR PIPES, DUCTS, ETC. UNLESS NOTED. CONTRACTOR SHALL OBTAIN PRIOR APPROVAL FOR INSTALLATION OF ANY WORK.

CAST-IN-PLACE CONCRETE

1. VERIFY ALL DIMENSIONS COORDINATE WITH ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION AND NOTIFY ARCHITECT AND OR ENGINEER OF ANY DISCREPANCIES.
2. ALL CONCRETE SHALL CONFORM TO THE AMERICAN CONCRETE INSTITUTED SPECIFICATION, ACI #301 AND ACI #318, LATEST EDITION.
3. THE MINIMUM 28 DAYS CYLINDER STRENGTH SHALL BE AS FOLLOWS:

SYSTEM	STRENGTH AT 28 DAYS	MAXIMUM SLUMP	MAXIMUM AGGREGATE
SLAB-ON-GRADE	3000 PSI	5"	1-1/2"

4. ALL CONDUIT OR PLUMBING LINES IN SLAB SHALL BE PLACED BELOW SLAB REINFORCING. ALL CONDUIT TO BE NO GREATER THAN 1" DIAMETER AND TO BE PLACED IN CENTER OF SLAB. NO PLUMBING LINES GRATER THAN 1" ALLOWED IN THE SLAB.
5. ALL OPENINGS IN SLAB (PER PIPING DRAWS, ETC.) SHALL BE SEALED WITH 1/2 SEALANT (SILICONE - LEVELING 2 PART POLYURETHANE)
6. THE VAPOR RETARDANT BELOW ALL SLAB AREAS SHALL BE 6 MIL POLYETHYLENE WITH ALL JOINTS LAPPED 12" CONTINUOUS AND SEALED. DROP VAPOR BARRIER DOWN THE SIDES OF ALL BEAM TRENCHES. DO NOT PLACE VAPOR BARRIER ACROSS TRENCH BOTTOM.
7. CURING COMPOUND SHALL BE PLACED WITHIN FOUR (4) HOURS AFTER CONCRETE HAS BEEN PLACED. CONCRETE SHALL BE MAINTAINED ABOVE 50 DEGREES F AND IN A MOIST CONDITION FOR AT LEAST THE FIRST SEVEN (7) DAYS AFTER PLACEMENT.
8. CONCRETE COVER FOR REINFORCING AS FOLLOWS:
   A. DRILLED PIERS, FOOTINGS AND OTHER FOUNDAL STRUCTURAL MEMBERS IN WHICH CONCRETE IS DEPOSITED AGAINST THE GROUND: 3"
   B. WHERE CONCRETE SURFACES AFTER REMOVAL OF FORMS ARE EXPOSED TO WEATHER OR GROUND:
      FOR BARS GREATER THAN #5 IN DIAMETER 2"
      FOR BARS #5 OR LESS IN DIAMETER 1-1/2"
   C. WHERE SURFACES ARE NOT DIRECTLY EXPOSED TO WEATHER OR GROUND:
      FOR SLAB ON GRADE (FROM TOP OF SLAB) 1-1/2"
      FOR BEAMS, COLUMNS AND WALLS 1-1/2"
      FOR JOISTS AND SLABS 3/4"
13. ANCHOR BOLTS, DOWELS, INSERTS, ETC. SHALL BE SECURELY TIED IN PLACE PRIOR TO PLACING CONCRETE.
14. REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR ALL MOLDS, GROOVES, REBLETS, ORNAMENTAL CLIPS, PIPES, CONDUITS, INSERTS, ETC. TO BE CAST IN CONCRETE. PROVIDE OVERSIZED SLEEVES FOR PLUMBING AND ELECTRICAL CONDUITS AND PIPES. NO PIPES OR DUCTS SHALL BE PLACED IN CONCRETE, FOOTINGS, OR SLAB UNLESS SPECIFICALLY DETAILED IN THESE PLANS, OR AS DIRECTED BY THE ENGINEER.
15. UTILITIES THAT PROJECT THROUGH SLAB FLOORS SHOULD BE DESIGNED WITH EITHER SOME DEGREE OF FLEXIBILITY OR WITH SLEEVES IN ORDER TO PREVENT DAMAGE TO THE LINES SHOULD VERTICAL MOVEMENT OCCUR.
16. CONCRETE TO BE CURED IN ACCORDANCE WITH ACI RECOMMENDATIONS. PROPOSED METHOD OF CURING TO BE COORDINATED WITH ENGINEER PRIOR TO CONCRETE PLACEMENT.

REINFORCING STEEL

1. ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS AND ALL ACCESSORIES UNLESS OTHERWISE NOTED, SHALL BE IN ACCORDANCE WITH THE ACI "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE", ACI #315 LATEST EDITION.
2. ALL REINFORCING BARS SHALL BE NEW BILLET STEEL AND SHALL CONFORM TO ASTM A615 GRADE 60 SPECIFICATIONS.
3. PROVIDE CORNER BARS TOP AND BOTTOM AT ALL BEAM CORNERS AND DEAD END BEAM INTERSECTIONS. BARS TO EQUAL SIZE AND QUANTITY OF THE NOTED BEAM STEEL. BARS SHALL LAP BEAM REINFORCEMENT 40 BAR DIAMETERS.
4. BARS DETAILED AS CONTINUOUS SHALL BE LAPPED 40 BAR DIAMETERS AT SPLICES.
5. EXTEND THE SLAB REINFORCING STEEL, PERPENDICULAR TO THE BEAM, TO THE TOP OUTSIDE REINFORCING BAR OF PERIMETER BEAMS. START THE SLAB REINFORCING STEEL, PARALLEL TO THE BEAM, NOT MORE THAN 6" FROM THE TOP INSIDE REINFORCING BAR OF PERIMETER BEAMS.
6. PROVIDE #4 "Z" BARS AT 12" ON CENTER WHERE THE SLAB STEPS DOWN MORE THAN 3". THE "Z" BARS AT 12" ON CENTER WHERE THE SLAB STEPS DOWN MORE THAN 3". THE "Z" BARS SHALL LAP THE MAIN SLAB REINFORCING STEEL 40 BAR DIAMETERS.

STRUCTURAL STEEL

1. MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITION OF THE AISC SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.
2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING ASTM DESIGNATIONS:

PLATES	A36 (FY = 36ksi)
STEEL TUBES (COLD FORMED HSS)	A500, GRADE B (FY = 46ksi)
STEEL PIPES	A53, GRADE B (FY = 35ksi)
STEEL ANGLES	ASTM A36, GRADE B (36ksi)
HOT ROLLED SHAPES	ASTM A572 (50ksi)

3. ALL STRUCTURAL STEEL SHALL BE FABRICATED, ERECTED AND PAINTED IN ACCORDANCE WITH THE SPECIFICATIONS FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS AS AMENDED TO DATE AND THE CODE OF STANDARD PRACTICE, LATEST EDITION AS ADOPTED BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION, AMENDED AS FOLLOWS:

   SECTION 4.2.1 DELETE FIRST TWO SENTENCES.
   SECTION 7.7 ALL REFERENCE TO OWNER SHALL BE CHANGED TO GENERAL CONTRACTOR.
   SECTION 7.6.4 THE CONTRACTOR TO DESIGN SHORES, JACK OR LOADS.
4. WELDING SHALL BE DONE IN ACCORDANCE WITH THE STANDARD CODE FOR ARC AND GAS WELDING IN BUILDING CONSTRUCTION, AS PUBLISHED BY THE AMERICAN WELDING SOCIETY, EXCEPT THAT ALL WELDING SHALL BE DONE BY THE ELECTRIC ARC PROCESS. ALL WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS AND SHALL CONFORM TO ANSI/AWS D1.1/M.
5. DETAILED AND OR SCHEDULED CONNECTIONS HAVE BEEN DESIGNED BY A STRUCTURAL ENGINEER. ANY CONNECTION NOT DETAILED OR SCHEDULED OR ALTERED FOR FABRICATION PURPOSES SHALL BE SIZED AND DETAILED BY FABRICATOR AND SHALL BE MARKED FOR ENGINEER'S VERIFICATION. FABRICATOR SIZED AND DETAILED CONNECTIONS SHALL SUPPORT CHANGE THE TOTAL UNIFORM LOAD CAPACITY SHOWN IN THE TABLES OF UNIFORM CONSTANTS, PART 2 OF THE AISC MANUAL OF STEEL CONSTRUCTION FOR THE GIVEN BEAM, SPAN AND GRADE OF STEEL SPECIFIED. THE EFFECT OF ANY CONCENTRATION LOADS MUST BE TAKEN INTO ACCOUNT.
6. SEE ARCHITECTURAL PLANS FOR MISCELLANEOUS STEEL ITEMS NOT INDICATED ON STRUCTURAL DRAWINGS. STEEL ITEMS SHOWN ON ARCHITECTURAL DRAWINGS AND NOT SPECIFIED ON THE STRUCTURAL DRAWINGS SHALL BE DESIGNED BY THE STEEL FABRICATOR. SEE DESIGN CRITERIA FOR LOADING.
7. ALL WELDED CONNECTIONS SHALL BE MADE USING 1/4" FILLET WELD, U.N.O. OR AS SPECIFIED ON THE PLANS.
8. ALL BOLTED CONNECTIONS SHALL BE MADE USING 3/4" DIAMETER HIGH STRENGHT BOLTS, ASTM A325, BEARING TYPE CONNECTION w/ WASHERS ASTM F436, U.N.O. ON DESIGN DRAWINGS. SPECIAL INSPECTION REQUIRED FOR ALL HIGH STRENGTH BOLTING. ALL NUTS SHALL BE PER ASTM A563.
9. ALL CONNECTION PLATES AND STIFFENERS SHALL BE MADE WITH 1/4" THICK PLATES, UNLESS OTHERWISE NOTED ON THE PLANS.
10. ALL STEEL (INCLUDING BOLTS) EXPOSED TO THE WEATHER SHALL BE HOT-DIPPED GALVANIZED. (INCLUDES STEEL THAT IS ONLY COVERED WITH PLASTER OR STUCCO).
11. ALL EXPOSED STEEL SHALL FOLLOW SECTION 10 OF THE CODE OF STANDARD PRACTICE OF AISC. SECTION 10 OF THE CODE ADDRESSES ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS).
12. CONNECTIONS SHALL BE PER HOLLOW STRUCTURAL SECTIONS, CONNECTION MANUAL BY AISC.
13. WHERE STEEL JOISTS PASS THROUGH CMU WALLS, PROVIDE HALF INCH GAP BETWEEN THE CMU AND THE STEEL JOIST. PROVIDE ELASTOMERIC MATERIAL BETWEEN THE STEEL JOIST AND CMU WALL.
14. ALL BEAMS NOT SHOWN SHALL BE W14x26. ALL COLUMNS NOT SHOWN SHALL BE HSS4x4x1/4.
15. PROVIDE 3000 LBS OF PED IRON ALLOWANCE. ALLOWANCE SHALL INCLUDE LABOR COSTS.
16. ALL STEEL MEMBERS, UNLESS NOTED OTHERWISE, SHALL BE PAINTED W/ ONE COAT OF SHOP PRIMER. DO NOT PRIME ITEMS TO BE EMBEDDED IN CONCRETE OR FIREPROOFED W/ SPRAY ON MATERIAL WITHOUT COORDINATION W/ THE ARCHITECT.
17. AFTER ERECTION, PRIME WELDS, ABRASIONS AND SURFACES NOT PRIMED, USE PRIMER CONSISTENT WITH SHOP COAT. GALVANIZED SURFACES (HOT DIPPED OR COLD) SHALL BE CLEANED AND PAINTED WITH "ZRC".
18. FIELD WELDS AND BOLTED CONNECTIONS SHALL BE VISUALLY INSPECTED BY A QUALIFIED INDEPENDENT INSPECTOR. THE INSPECTOR SHALL PROVIDE A WRITTEN REPORT TO THE STRUCTURAL ENGINEER.
19. THE CONTRACTOR SHALL REVIEW AND ANNOTATE SHOP DRAWINGS BEFORE SUBMITTING THEM TO THE ARCHITECT/ENGINEER FOR REVIEW. THE CONTRACTOR SHALL ALLOW ARCHITECT/ENGINEER 10 WORKING DAYS FOR REVIEW OF SHOP DRAWINGS.

LIGHTWEIGHT STEEL FRAMING (STEEL STUDS)

1. THE GENERAL CONTRACTOR, PROJECT SUPERINTENDENT AND THE LIGHTWEIGHT STEEL FRAMING FOREMAN SHALL MEET WITH THE STRUCTURAL ENGINEER PRIOR TO THE START OF LIGHTWEIGHT STEEL FRAMING WORK TO REVIEW PROJECT REQUIREMENTS AND PROCEDURES.
2. ALL STUDS AND ACCESSORIES SHALL BE OF THE TYPE, SIZE, GAUGE AND SPACING SHOWN ON THE DRAWINGS. STUD MANUFACTURING OF ALL MEMBERS AND ACCESSORIES SHALL COMPLY WITH ASTM C955.
3. ALL STUDS AND JOIST MEMBERS 16 GAUGE AND HEAVIER SHALL BE FORMED FROM STEEL CORRESPONDING TO A TYPE LISTED IN THE AISI "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", WITH A MINIMUM YIELD STRENGTH OF 50 KSI UNLESS SPECIFICALLY NOTED OTHERWISE.
4. ALL 18 GAUGE AND LIGHTER MEMBERS, TRACK, BRIDGING, AND ACCESSORY ITEMS SHALL BE FORMED FROM STEEL "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS", WITH A MINIMUM YIELD STRENGTH OF 33 KSI UNLESS SPECIFICALLY NOTED OTHERWISE.
5. ALL FRAMING PRODUCTS SHALL BE FORMED FROM STEEL POSSESSING A COATING CORRESPONDING TO THE MINIMUM REQUIREMENTS OF ASTM 653, G90.
6. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY FOR ATTACHMENT TO PERPENDICULAR ENMEBRS, OR, AS REQUIRED, FOR AN ANGULAR FIT AGAINST ABUTTING MEMBERS.
7. ALL FIELD CUTTING OF STUDS MUST BE DONE BY SAWING OR SHEARING. TORCH CUTTING OF COLD MEMBERS IS UNACCEPTABLE.
8. WHEN REQUIRED FOR BRIDGING PURPOSES, THE FRAMING FABRICATOR IS TO ENSURE PUNCH OUT ALIGNMENT WHEN ASSEMBLING FRAMING AND FIELD CUTTING STUDS TO LENGTH. REFER TO TYPICAL STUD/TRACK CONNECTION AND INDEXING DETAILS.
9. NO SPLICES IN STUDS, JOIST, OR OTHER LOAD CARRYING MEMBERS MAY BE MADE WITHOUT PRIOR ENGINEERING REVIEW AND SPECIFIC DETAILS FOR ANY SUCH SPLICE(S).
10. NO NOTCHING OR COPING OF STUDS, JOISTS OR RAFTERS IS ALLOWED UNLESS STATED WITHIN THESE PLANS.
11. WHERE SPLICING OF WALL TRACK IS NECESSARY BETWEEN STUDS, A PIECE OF STUD SHALL BE PLACED IN THE ADJOINING TRACK SECTION AND FASTENED TO THE TRACK FLANGES AT BOTH SIDES OF THE WALL OR THE TRACKS AND SHALL BE BUTTED TIGHT TOGETHER AND FASTENED TO THE STRUCTURE EITHER SIDE OF THE JOINT.
12. FOR ALL TRACKS USED IN COMPOSITE MEMBERS SUCH AS TRUSSES, POSTS, HEADERS AND JAMBS, THE TRACK MUST BE INSTALLED AS A SINGLE PIECE; NO SPLICING IS PERMITTED.
13. ALL SCREWS SHALL BE OF THE DIAMETER AND SIZE INDICATED ON THE DRAWINGS, AND SHALL BE MANUFACTURED AND TESTED BY ITW BUILDEX OR EQUIVALENT. A MINIMUM 1/2 INCH EDGE DISTANCE AND 1/2 INCH SCREW SPACING IS REQUIRED UNLESS NOTED OTHERWISE ON THE DRAWINGS. SCREW PENETRATION THROUGH JOINED MATERIALS SHALL NOT BE LESS THAN THREE EXPOSED THREADS. SCREW DESIGN IS BASED ON A TYPE "D" SCREW LINO.
14. ALL POWDER ACTUATED FASTENERS (PAF) SUBJECTED TO TENSION SHALL HAVE A 15MM (MIN) DIAMETER STEEL WASHER. THE FOLLOWING MINIMUM EDGE DISTANCES AND FASTENER SPACING APPLY.

SPACING APPLY:

	EDGE DISTANCE (in.)		MINIMUM SPACING (in.)		PENETRATION (in.)	
PIN DIAMETER (in.)	STEEL	CONCRETE	STEEL	CONCRETE	STEEL	CONCRETE
0.157	1/2	3	1	4	FULL	VARIES

ROOF METAL DECK

1. SHEET METAL:
   MATERIAL: ASTM A446, GRADE A
   GALVANIZING: G90 ZINC COATED, ACCORDING TO ASTM A525
   LOCATION: TYPICAL
2. ATTACHMENT:
   DECK PROFILE: B
   PROFILE DEPTH: 1.5"
   GAUGE: 24
   MAX SPAN: 6'-0"
3. INSTALL DECK ENDS OVER SUPPORTING FRAMING WITH A MINIMUM END BEARING OF 1.5" WITH END JOINTS LAPPED AT A MINIMUM OF TWO INCHES AND SHALL OCCUR OVER SUPPORTS.
4. SCREWS MUST BE INSTALLED USING PROPERLY CALIBRATED TOOLS TO AVOID OVERDRIVING WHICH CAN STRIP THE THREADS AT SIDE LAPS OR SEVER THE SCREW WHEN IT IS PLACED INTO HEAVIER SUBSTRATE.
5. DECK UNITS SHALL BE 3 OR MORE SPANS AND SHALL BE ATTACHED TO THE STRUCTURAL SUPPORT.
6. PROVIDE CLOSURE OVER WALLS AND UNDER DECKS, ALSO OVER STEEL BEAMS AND DECKS.
7. PROVIDE FILLER SHEETS AS REQUIRED.
8. AT VALLEYS, RIDGES AND WHERE DECK CHANGES DIRECTIONS, PROVIDE 9" WIDE x 1/4" THICK CONTINUOUS FLAT PLATES. (BEND AS REQUIRED).
9. BUILDING ROOF CORNERS AND PERIMETERS SHALL BE FASTENED PER NOT 2 ABOVE OR PER ROOF SYSTEM REQUIREMENTS, WHICHEVER HAS THE MOST STRINGENT REQUIREMENTS.

FASTENERS

1. CAST-IN-PLACE AND POST-INSTALLED ANCHORS SHALL BE PER ANCHOR DIAMETER AND EMBEDMENT DEPTH NOTED ON THE DRAWINGS. POST-INSTALLED ANCHORS SHALL BE UTILIZED ONLY WHERE SPECIFIED. ALL ANCHORS SHALL BE HOT-DIPPED GALVANIZED PER ASTM A153.
2. ALL ANCHORS NOTED BELOW SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. CONTRACTOR SHALL CONTACT MANUFACTURER'S REPRESENTATIVE FOR THE INITIAL TRAINING AND INSTALLATION OF ANCHORS, AND FOR PRODUCT RELATED QUESTIONS AND AVAILABILITY.
3. SPECIAL INSPECTIONS SHALL BE PROVIDED FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE EVALUATION REPORT NOTED BELOW. SPECIAL INSPECTIONS SHALL BE PERFORMED BY AN INDEPENDENT TESTING LABORATORY PERFORMING GAGE SERVICES ON THE PROJECT.
4. EXPANSION BOLTS (EB) IN CONCRETE/CMU SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193, ACCEPTABLE PRODUCTS:
   - KWIK BOLT III (ICC-ES ESR-2302) BY HILTI (CONCRETE)
   - KWIK BOLT TZ (ICC-ES ESR-1917) BY HILTI (MASONRY)
   - STRONG-BOLT 2 (ICC-ES ESR-3037) BY SIMPSON STRONG-TIE (CONCRETE)
   - WEDGE-ALL ANCHOR (ICC-ES ESR-1396) BY SIMPSON STRONG-TIE (MASONRY)
5. HEAVY DUTY SLEEVE ANCHORS IN CONCRETE/CMU SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193. EXPANSION BOLTS (EB) SHALL NOT BE SUBSTITUTED FRO SLEEVE ANCHORS WITHOUT PRIOR WRITTEN APPROVAL BY THE STRUCTURAL ENGINEER. ACCEPTABLE PRODUCTS:
   - HSL-3 (ICC-ES ESR-1545) BY HILTI (CONCRETE)
6. SCREW ANCHORS IN CONCRETE SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193, ACCEPTABLE PRODUCTS:
   - KWIK HUS-EZ (ICC-ES ESR-3027) BY HILTI (CONCRETE)
   - KWIK HUS-EZ (ICC-ES ESR-3056) BY HILTI (MASONRY)
   - TITEN HD (ICC-ES ESR-2713) BY SIMPSON STRONG-TIE (CONCRETE)
   - TAPCON ANCHOR (ICC-ES ESR-1671) (MASONRY)
   - POWERS WEDGE BOLT (ICC-ES ESR-1671) (MASONRY)
7. UNDERCUT ANCHORS IN CONCRETE SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193, ACCEPTABLE PRODUCTS:
   - HDA (ICC-ES ESR-1546) BY HILTI (CONCRETE)
   - TORQ CUT (ICC-ES ESR-2769) BY SIMPSON STRONG-TIE (CONCRETE)
8. POWDER ACTUATED FASTENERS IN CONCRETE/CMU SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC192, ACCEPTABLE PRODUCTS:
   - X-U (ICC-ES ESR-2269) BY HILTI (CONCRETE/MASONRY)
   - POWDER ACTUATED FASTENERS (ICC-ES ESR-2024) BY SIMPSON STRONG-TIE (CONCRETE/MASONRY)
9. ADHESIVE ANCHORS AND CONCRETE/CMU SHALL BE TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308, ACCEPTABLE PRODUCTS:
   - HIT-RE 500-SD (ICC-ES ESR-2322) BY HILTI (CONCRETE)
   - HIT-HP 150 MAX (ICC-ES ESR-1967) BY HILTI (MASONRY)
   - SET-XP (ICC-ES ESR-2508) BY SIMPSON STRONG-TIE (CONCRETE)
   - SET (ICC-ES ESR-1772) BY SIMPSON STRONG-TIE (MASONRY)
10. J-BOLTS SHALL BE FABRICATED FROM ASTM A36/A307 ROD BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. EXPANSION BOLTS/SLEEVE ANCHORS SHALL NOT BE SUBSTITUTED FOR J-BOLTS WITHOUT PRIOR WRITTEN APPROVAL BY THE STRUCTURAL ENGINEER.

ATLAS ENGINEERING CONSULTANTS
TBPE FIRM NO. F-3017

PRELIMINARY FOR BIDDING PURPOSES ONLY

THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY ALAN GUSTAVO LOPEZ, P.E. ON 11/09/21. ALTERATION OF A SEALED DOCUMENT IS A VIOLATION OF THE TEXAS ENGINEERING PRACTICE ACT. THE STATE OF TEXAS REGISTERED PROFESSIONAL ENGINEER

PROJECT: PROPOSED BUILDING ADDTION KINGSVILLE, TEXAS

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO. ST21-083

DATE 05/18/2021

DRAWN BY L.C.

SCALE AS NOTED

GENERAL NOTES

S1.0

GENERAL NOTES

GEOTECHNICAL ENGINEERING REPORT

THESE PLANS AND GENERAL NOTES HAVE BEEN PREPARED IN ACCORDANCE WITH THE DESIGN RECOMMENDATIONS PRESENTED BY ATLAS ENGINEERING CONSULTANTS GEOTECHNICAL ENGINEERING REPORT NO. 021-018, DATED JUNE 13, 2021.

THE FOLLOWING FOUNDATION DESIGN CRITERIA WAS USED IN THE FOUNDATION DESIGN:

MINIMUM GRADE BEAM WIDTH	12 INCHES
MINIMUM WIDENED SECTION WIDTH:	24 INCHES
ALLOWABLE SOIL BEARING CAPACITY:	2,000 PSF (CONTINUOUS FOOTING) 3,000 PSF (SPREAD FOOTING)

EXCAVATIONS AND BACKFILL REQUIREMENTS

THE FOLLOWING SITE PREPARATION IS REQUIRED PRIOR TO CONSTRUCTION:

1. EXCAVATE AND REMOVE ALL VEGETATION AND DELETERIOUS MATERIALS FROM THE SURFACE. EXCAVATE TO A DEPTH OF AT LEAST 4 FEET BELOW THE SURFACE AND REMOVE FROM SITE OR FOR USE IN LANDSCAPING.
2. THE EXCAVATION SHOULD EXTEND A MINIMUM OF FIVE (5) FEET BEYOND THE PERIMETER OF THE BUILDING. POSITIVE DRAINAGE FROM THE STRUCTURE SHOULD BE PROVIDED.
3. THE EXPOSED SUBGRADE SOILS SHOULD BE COMPACTED TO 95 PERCENT OF THE STANDARD PROCTOR (ASTM D698) FOR A DEPTH OF 8 INCHES BELOW THE EXPOSED SURFACE. THE MOISTURE CONTENT OF THE COMPACTED SUBGRADE SOILS SHOULD BE WITHIN THE RANGE OF OPTIMUM TO 3 PERCENT ABOVE THE OPTIMUM MOISTURE CONTENT.
4. AFTER SUBGRADE PREPARATION AND OBSERVATION HAVE BEEN COMPLETED, A MINIMUM OF 3.5 FEET OF SELECT FILL MEETING THE SPECIFICATIONS PRESENTED BELOW, SHOULD BE PLACED BACK ON TOP OF THE PREPARED SUBGRADE SOILS. THE SOILS SHOULD BE PLACED IN MAXIMUM 8-INCH LOOSE LIFTS AND COMPACTED TO A MINIMUM OF 95 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D698.
5. THE MOISTURE CONTENT, AS DETERMINED BY ASTM D698, SHOULD BE MAINTAINED UNTIL CONSTRUCTION IS COMPLETE. EACH LIFT OF COMPACTED FILL SHOULD BE TESTED BY A TESTING LABORATORY PRIOR TO PLACEMENT OF SUBSEQUENT LIFTS.
6. PROPERLY COMPACTED AND TESTED SELECT FILL SHOULD BE USED TO ACCOMMODATE RAISE IN GRADE TO ACHIEVE THE DESIRED FINISHED FLOOR ELEVATION.
7. SELECT FILL SHOULD HAVE A MAXIMUM LIQUID LIMIT OF 40 PERCENT, A PLASTICITY INDEX BETWEEN 7 AND 15 PERCENT AND A MAXIMUM PARTICLE SIZE NOT EXCEEDING 4 INCHES OR ONE-HALF THE LOOSE LIFT THICKNESS WHICHEVER IS SMALLER.
8. SAMPLES OF PROPOSED SELECT FILL SHALL BE FURNISHED TO THE TESTING LABORATORY 7 DAYS PRIOR TO INSTALLATION IN ORDER TO PERMIT TIME FOR SPECIFICATION COMPLIANCE INSPECTION AND REVIEW BY THE GEOTECHNICAL ENGINEER.
9. LABORATORY MOISTURE-DENSITY CURVES SHALL BE DEVELOPED FOR SUBGRADE AND SELECT FILL. PROCTOR CURVES AND FIELD DENSITY TESTS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. A MINIMUM OF ONE (1) IN PLACE DENSITY TEST PER 5,000 SQUARE FEET OF SLAB AREA SHALL BE TAKEN ON EACH LIFT DURING PLACEMENT OF SELECT FILL. DENSITY REPORTS SHALL BE TRANSMITTED TO THE ENGINEER WITHIN 3 DAYS AFTER TESTS ARE MADE.
10. GRAIN SIZE ANALYSIS AND ATTERBERG LIMITS TEST SHALL BE PERFORMED DURING FILL PLACEMENT AT A RATE OF ONE TEST PER 5,000 CUBIC YARDS OF FILL BROUGHT TO THE SITE. SAMPLES FOR TEST SHALL BE TAKEN FROM JOBSITE MATERIALS.
11. SITE SHALL BE GRADED SO THAT WATER DOES NOT POND WITHIN 10 FEET OF THE PERIMETER FOUNDATION BEAM DURING OR AFTER CONSTRUCTION. THE SLOPE OF THE GROUND SURFACE AWAY FROM THE STRUCTURE SHOULD BE A MINIMUM OF 5% FOR A DISTANCE OF AT LEAST 10 FEET. ELEVATION OF GROUND SURFACE ADJACENT TO THE FOUNDATION SHOULD BE AT LEAST 6 INCHES BELOW FINISH FLOOR.
12. FOUNDATION CONCRETE SHALL NOT BE PLACED ON SELECT FILL SOILS THAT HAVE BEEN DISTURBED BY RAINFALL OR WATER SEEPAGE. IF BEARING SOILS ARE SOFTENED BY WATER INTRUSION OR BY DESICCATION, THE UNSUITABLE SOILS SHALL BE REMOVED FROM THE FOUNDATION EXCAVATION AND BE REPLACED WITH PROPERLY COMPACTED SELECT FILL PRIOR TO PLACEMENT OF FOUNDATION CONCRETE. ALL SOIL REMOVAL AND REPLACEMENT COSTS, INCLUDING ASSOCIATED COSTS TO REMOVE AND REINSTALL REINFORCEMENT AND VAPOR RETARDER MATERIALS, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR. DEPTH OF SOIL REMOVAL AND RECOMPACTION REQUIREMENTS SHALL BE COORDINATED WITH THE GEOTECHNICAL ENGINEER.

METAL BUILDING SYSTEMS

1. PRE-ENGINEERED METAL BUILDING SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS AND HAVING THREE (3) OR MORE YEARS EXPERIENCE IN THE DESIGN OF THE TYPE OF THE BUILDING INDICATED ON THE CONTRACT DOCUMENTS.
2. THE METAL BUILDING AND COMPONENTS SHALL BE DESIGNED TO CARRY ITS OWN WEIGHT PLUS ALL SUPERIMPOSED DEAD AND LIVE LOADS INCLUDING ELECTRICAL AND ARCHITECTURAL LOADS. VERIFY ALL LOADS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL PLANS.
3. VERIFY ALL DIMENSIONS AND SITE CONDITIONS PRIOR TO DESIGN, FABRICATION OR ERECTION OF PRE-ENGINEERED BUILDINGS.
4. PRE-ENGINEERED BUILDING FRAMES AND THE CONNECTION OF FRAME TO THE FOUNDATION IS TO BE DESIGNED BY OTHERS AND IS NOT THE RESPONSIBILITY OF ATLAS ENGINEERING CONSULTANTS (ATLAS). CONTRACTOR SHALL COORDINATE THE CONNECTION OF THE BUILDING FRAME WITH THE SUPPLIER PRIOR TO CONSTRUCTION.
5. THIS FOUNDATION HAS BEEN DESIGNED USING ASSUMED REACTIONS FROM THE PRE-ENGINEERED BUILDING COMPONENTS AND IS FOR BID PURPOSES ONLY. THE CONTRACTOR SHALL SUBMIT BASE CONNECTION DETAILS (SIZE AND THICKNESS BASE PLATE AND DIAMETER AND LENGTH ANCHOR BOLTS) AND REACTIONS OF THE BUILDING FRAMES TO THE ENGINEER PRIOR TO CONSTRUCTION SO THE DESIGN ASSUMPTIONS CAN BE VERIFIED. DEPTH OF ANCHOR BOLTS SHALL BE SUFFICIENT TO PREVENT CONICAL SHEAR OF THE CONCRETE FOUNDATION.
6. PRE-ENGINEERED METAL BUILDING ANCHOR BOLTS SHALL BE DESIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF TEXAS AND HAVING THREE (3) OR MORE YEARS EXPERIENCE IN THE DESIGN OF THE TYPE OF BUILDING INDICATED ON THE CONTRACT DOCUMENTS.
7. METAL BUILDING SUPPLIER SHALL PROVIDE AND SUBMIT FOR REVIEW ALL DESIGN CALCULATIONS AND DRAWINGS. ALLOW TWO (2) WEEKS FOR REVIEW OF SHOP DRAWINGS.
8. ANY ADDITIONAL COST OF FOUNDATION WORK REQUIRED BY REVISIONS OF THE FOUNDATION DESIGN AFTER PRE-ENGINEERED BUILDING REACTIONS ARE SUBMITTED SHALL BE BY THE CONTRACTOR.
9. METAL ROOF DOES NOT PROVIDE LATERAL BRACING FOR THE PURLINS. BRIDGING SHALL BE DESIGNED AND SUPPLIED BY THE PURLIN MANUFACTURER.
10. REFER TO MECHANICAL DRAWINGS FOR ROOF SUPPORTED HVAC UNITS AND PROVIDE SUPPORT FOR ADDITIONAL LOADS REQUIRED.
11. MAXIMUM PURLIN SPACING SHALL BE 4'-0" O.C. WITH A MAXIMUM ALLOWABLE TOTAL DEFLECTION OF L/240.
12. PRE-ENGINEERED BUILDING MANUFACTURER SHALL PROVIDE ADDITIONAL FRAMING REQUIRED TO SUPPORT THE WEIGHT OF MECHANICAL UNITS AND PROVIDE PROPER SERVICEABILITY OF SUSPENDED MECHANICAL UNITS, MECHANICAL DUCTWORK, LIGHT FIXTURES, AND ALL OTHER SUSPENDED ITEMS AND ITEMS SUPPORTED ON TOP OF ROOF.
13. DETAILS SHALL BE INCLUDED WHICH CLEARLY DETAIL RIGID FRAME BASE, HAUNCH, RIDGE, PLATE CONNECTIONS, AND OTHER MEMBER-TO-MEMBER CONNECTIONS.
14. WIND LOAD DESIGN SHALL INDICATE METHOD OF TRANSFERRING FORCES:
    A. END WALL WIND LOAD TO SIDE WALL FOUNDATIONS.
    B. AT END BAY SIDE WALL WIND LOAD TO END WALL FOUNDATIONS. CALCULATIONS SHALL SHOW HOW WIND LOAD IS TRANSFERRED TO EAVE STRUT.
15. PORTAL MOMENT FRAMES SHALL BE USED TO RESIST HORIZONTAL WIND FORCES. DESIGN OF ALL CONNECTIONS SHALL BE CLEARLY INDICATED.
16. DESIGN OF HORIZONTAL CROSS-BRACING IN PLANE OF ROOF FRAMING SHALL BE COMPLETE AND SHALL INDICATE METHOD OF TRANSFERRING TRIBUTARY WIND LOAD TO RIGID FRAMES OR THE SIDE WALL PORTAL FRAMES.
17. ALL COLUMN BASE PLATES SHALL BE SET AND GROUTED UNDER FOR FULL CONTACT BEARING.
18. ALL BASES FOR THE COLUMNS SHALL BE "PINNED" AND NOT ASSUMED AS FIXED. NO MOMENT FORCES SHALL BE TRANSFERRED INTO THE BUILDING FOUNDATION.
19. PROVIDE BUILDING CROSS SECTIONS AND ELEVATIONS WHICH CLEARLY SHOW THE PRIMARY STRUCTURAL RIGID MOMENT FRAME, PORTAL MOMENT FRAME, END WALL POST AND GIRTS, INTERIOR COLUMNS, AND OTHERS STRUCTURAL MEMBERS THAT ARE TO BE USED ON THE SUBMITTED BUILDING. SIZE OF ALL STANDARD MISC MEMBERS AND OF ALL WEB AND FLANGE SECTIONS USED IN BUILT-UP MEMBERS.
20. DESIGN AND MEMBERS FOR FRAMED OPENING SHALL BE PROVIDED AS PART OF THE METAL BUILDING DESIGN NOTED AS WELL AS BOLTS AND WELDING.
21. ALL STEEL (INCLUDING BOLTS) EXPOSED TO THE WEATHER SHALL BE HOT DIPPED GALVANIZED. (INCLUDES STEEL THAT IS ONLY COVERED WITH PLASTER OR STUCCO). SEE ARCHITECTURAL PLANS IF STRICTER REQUIREMENTS ARE REQUIRED.

DESIGN CRITERIA

1. DESIGN LOADS, STRUCTURAL ANALYSIS AND PREPARATIONS OF STRUCTURAL MEMBERS ARE BASED ON THE FOLLOWING:

• CODE	2018 IRC
• WIND DESIGN	ASCE 07 - 16
• ROOF LIVE LOAD	20 PSF (REDUCIBLE)
• ROOF DEAD LOAD A. MBS B. COLLATERAL LOAD	 SELF WEIGHT PSF 10 PSF
• WIND LOAD	149 MPH
• RISK CATEGORY	IV
• IMPORTANCE FACTOR:	1.15
• WIND EXPOSURE	C
• INT.PRES. COEFFICIENT(GCpi)	+/-0.18
• Kzt	1.0
• Kd	0.85
• COMPONENT AND CLADDING	SEE FRAMING PLAN
• SITE CLASS	D
• SS	0.063g
• S1	0.017g
• SDS	0.101
• SD1	0.049

STRUCTURAL FIELD OBSERVATIONS

1. JOB SITE OBSERVATIONS BY THE PROFESSIONAL ENGINEER OR HIS AUTHORIZED REPRESENTATIVE SHALL CONSIST OF VISUAL OBSERVATION OF MATERIALS, EQUIPMENT OR CONSTRUCTION WORK FOR THE PURPOSE OF ASCERTAINING THAT THE WORK IS IN SUBSTANTIAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND WITH THE DESIGN INTENT. SUCH OBSERVATIONS SHALL NOT BE RELIED UPON BY OTHERS AS ACCEPTANCE OF THE WORK NOR SHALL IT BE CONSTRUED TO RELIVE THE CONTRACTOR IN ANY WAY FROM HIS OBLIGATIONS AND RESPONSIBILITIES UNDER THE CONSTRUCTION CONTRACT, SPECIFICALLY, BUT WITHOUT LIMITATION, OBSERVATIONS BY THE DESIGN PROFESSIONAL SHALL NOT REQUIRE THE DESIGN PROFESSIONAL TO ASSUME RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTION, NOR FOR SAFETY ON THE JOB SITE.
2. NOTIFY ENGINEER 24 HOURS IN ADVANCE WHEN A STRUCTURAL OBSERVATION IS REQUIRED. SPECIAL INSPECTIONS INDEPENDENT OF THE CONTRACTOR, THE ARCHITECT, OR THE ENGINEER, SHALL BE PROVIDED BY A SPECIAL INSPECTOR EMPLOYED BY THE OWNER ACCORDING TO CHAPTER 17 OF THE IBC 2012. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE CONTRACT DOCUMENTS. THE SPECIAL INSPECTOR SHALL SEND WRITTEN REPORTS TO THE OWNER, THE ARCHITECT, THE ENGINEER, AND THE CONTRACTOR. THE REPORTS SHALL INDICATE IF WORK INSPECTED WAS DONE IN CONFORMANCE WITH THE CONTRACT DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE SPECIAL INSPECTOR SHALL BRING THE DISCREPANCIES TO THE ATTENTION OF THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. THE SPECIAL INSPECTION WORK WAS, TO THE BEST OF THEIR KNOWLEDGE, IN OR NOT IN CONFORMANCE WITH THE DRAWINGS, SPECIFICATIONS AND APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC 2012.

TABLE 1704.7
REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	—	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	—	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	—	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	—
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	—	X

TABLE 1704.4
REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	—	X
2. INSPECTION OF REINFORCING STEEL, WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 6B.	—	—
3. INSPECTION OF BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED OR WHERE STRENGTH DESIGN IS USED.	X	---
4. INSPECTION OF ANCHORS INSTALLED IN HARDENED CONCRETE.	—	X
5. VERIFYING USE OF REQUIRED DESIGN MIX.	—	X
6. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	—
7. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	—
8. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	—	X
9. INSPECTION OF PRESTRESSED CONCRETE:		
A. APPLICATION OF PRESTRESSING FORCES.	X	
B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-RESISTING SYSTEM.	X	---
10. ERECTION OF PRECAST CONCRETE MEMBERS.	—	X
11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POSTTENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	—	X
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	—	X

TABLE 1704.3
REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:		
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	—	X
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	—	X
2. INSPECTION OF HIGH-STRENGTH BOLTING:		
A. SNUG-TIGHT JOINTS.	—	X
B. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING, TWIST-OFF BOLT OR DIRECT TENSION INDICATOR METHODS OF INSTALLATION.	—	X
C. PRETENSIONED AND SLIP-CRITICAL JOINTS USING TURN-OF-NUT WITHOUT MATCHMARKING OR CALIBRATED WRENCH METHODS OF INSTALLATION.	X	—
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:		
A. FOR STRUCTURAL STEEL, IDENTIFICATION MARKINGS TO CONFORM TO AISC 360.	—	X
B. FOR COLD-FORMED STEEL, IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	—	X
C. MANUFACTURER'S CERTIFIED TEST REPORTS.	—	X
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:		
A. IDENTIFICATION MARKINGS TO CONFORM TO AWS SPECIFICATION IN THE APPROVED CONSTRUCTION DOCUMENTS.	—	X
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	—	X
5. INSPECTION OF WELDING:		
A. STRUCTURAL STEEL AND COLD-FORMED STEEL DECK:		
1. COMPLETE AND PARTIAL JOINT PENETRATION GROOVE WELDS.	X	—
2. MULTIPASS FILLET WELDS.	X	—
3. SINGLE-PASS FILLET WELDS < 5/16"	X	—
4. PLUG AND SLOT WELDS.	X	—
5. SINGLE-PASS FILLET WELDS ≤ 5/16"	—	X
6. FLOOR AND ROOF DECK WELDS.	—	X
B. REINFORCING STEEL:		
1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL, OTHER THAN ASTM A706.	—	X
2. REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCEMENT.	X	—
3. SHEAR REINFORCEMENT.	X	—
4. OTHER REINFORCING STEEL.	—	X
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE:		
A. DETAILS SUCH AS BRACING AND STIFFENING.	—	X
B. MEMBER LOCATIONS.	—	X
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	—	X

ATLAS ENGINEERING CONSULTANTS
TBPE F-10190, TBPELS F-10191
2100 Ranch Road 620 South, Suite 100
Austin, Texas 78738
www.atlasengineering.com

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PROJECT: PROPOSED BUILDING ADDTION KINGSVILLE, TEXAS

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO. ST21-083
DATE 05/18/2021
DRAWN BY L.C.
SCALE AS NOTED
GENERAL NOTES
S1.1

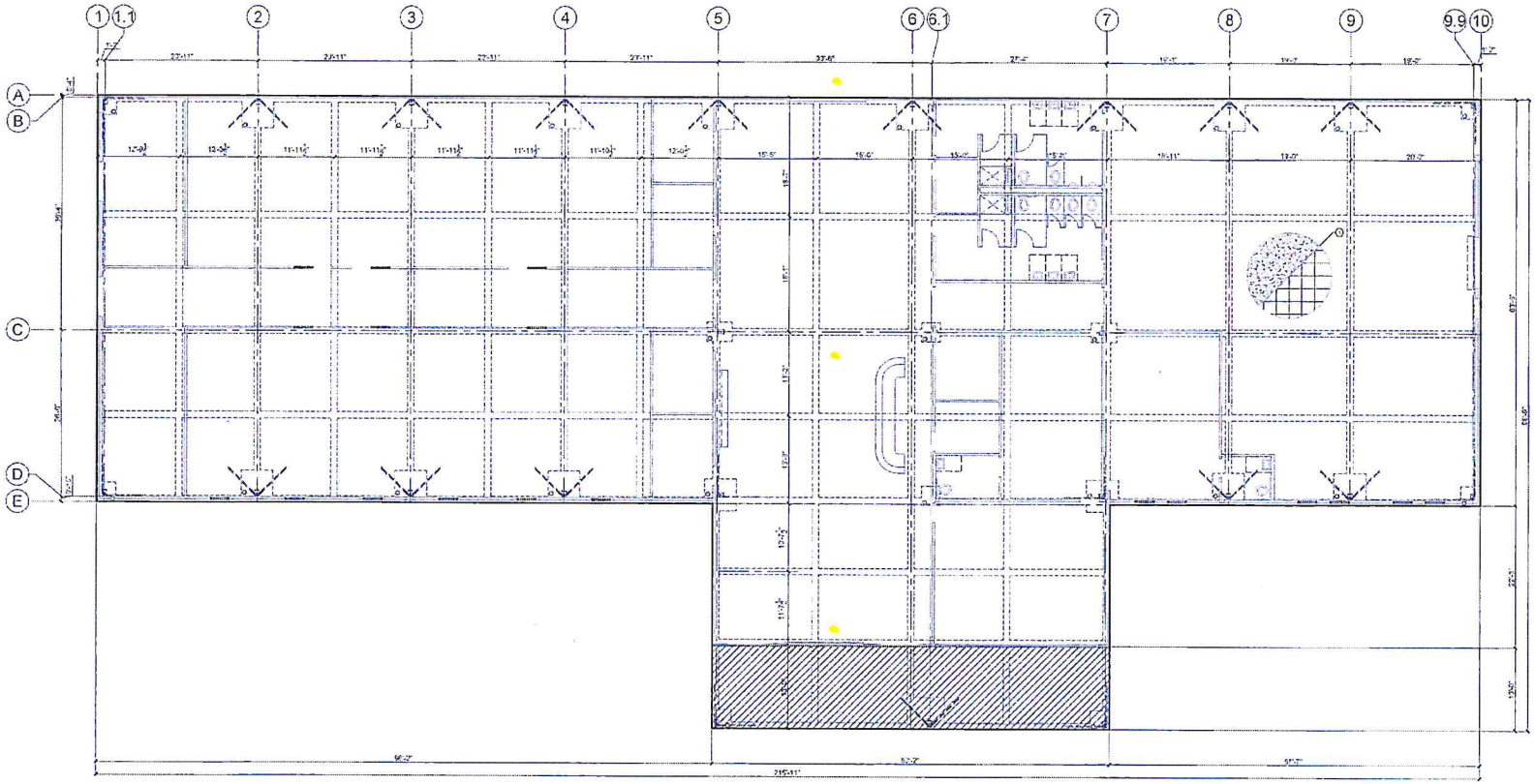
FOOTING SCHEDULE				
TYPE	DEPTH (FT)	SIZE (FT)	REINFORCEMENT	DETAIL
1	4	5 x 5	#5 @12 O.C. E.W. TOP & BOT.	4/SD1
2	4	3 x 3	#5 @12 O.C. E.W. TOP & BOT.	4/SD1
3	4	5 x 5	#5 @12 O.C. E.W. TOP & BOT.	4/SD1
4	4	5 x 5	#5 @12 O.C. E.W. TOP & BOT.	4/SD1
5	4	3 x 3	#5 @12 O.C. E.W. TOP & BOT.	4/SD1

NOTES:

1. SEE SHEET S1.0 FOR CONCRETE GENERAL NOTES.
2. SEE SHEET SD1 FOR TYPICAL DETAILS.
3. CONTRACTOR RESPONSIBLE FOR THE LOCATION OF ALL FLOOR DRAINS AND PIPING WHETHER OR NOT THEY ARE NOTED ON THESE STRUCTURAL DRAWINGS.
4. CONTRACTOR/SUBCONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL DIMENSIONS WITH ARCHITECTURAL PLANS BEFORE COMMENCING ANY WORK. THE CONTRACTOR/SUBCONTRACTOR SHALL REPORT ANY DISCREPANCIES TO THE ARCHITECT/ENGINEER BEFORE THE WORK HAS BEGUN.
5. REFER TO ARCHITECTURAL PLANS FOR ADDITIONAL DIMENSIONS NOT SHOWN.
6. TOP OF PIER PER ARCHITECTURAL PLANS.

PLAN NOTES:

1. INDICATES 5" THICK CONCRETE SLAB WITH #4 BARS AT 12" O.C. MID-DEPTH OF SLAB OVER 15 MIL VAPOR BARRIER OVER APPROVED COMPACTED FILL.
2. MINIMUM 3000 PSI COMPRESSIVE STRENGTH @ 28 DAYS AND MAX 5" SLUMP.
3. REFER TO GENERAL NOTES FOR EXCAVATION AND BACKFILL REQUIREMENTS.
4. VERIFY ALL SLAB DEPRESSIONS WITH ARCHITECTURAL DRAWINGS FOR EXTENT AND LOCATION.



FOUNDATION PLAN
SCALE: 1/8" = 1'

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TBPE FIRM NO. 12052
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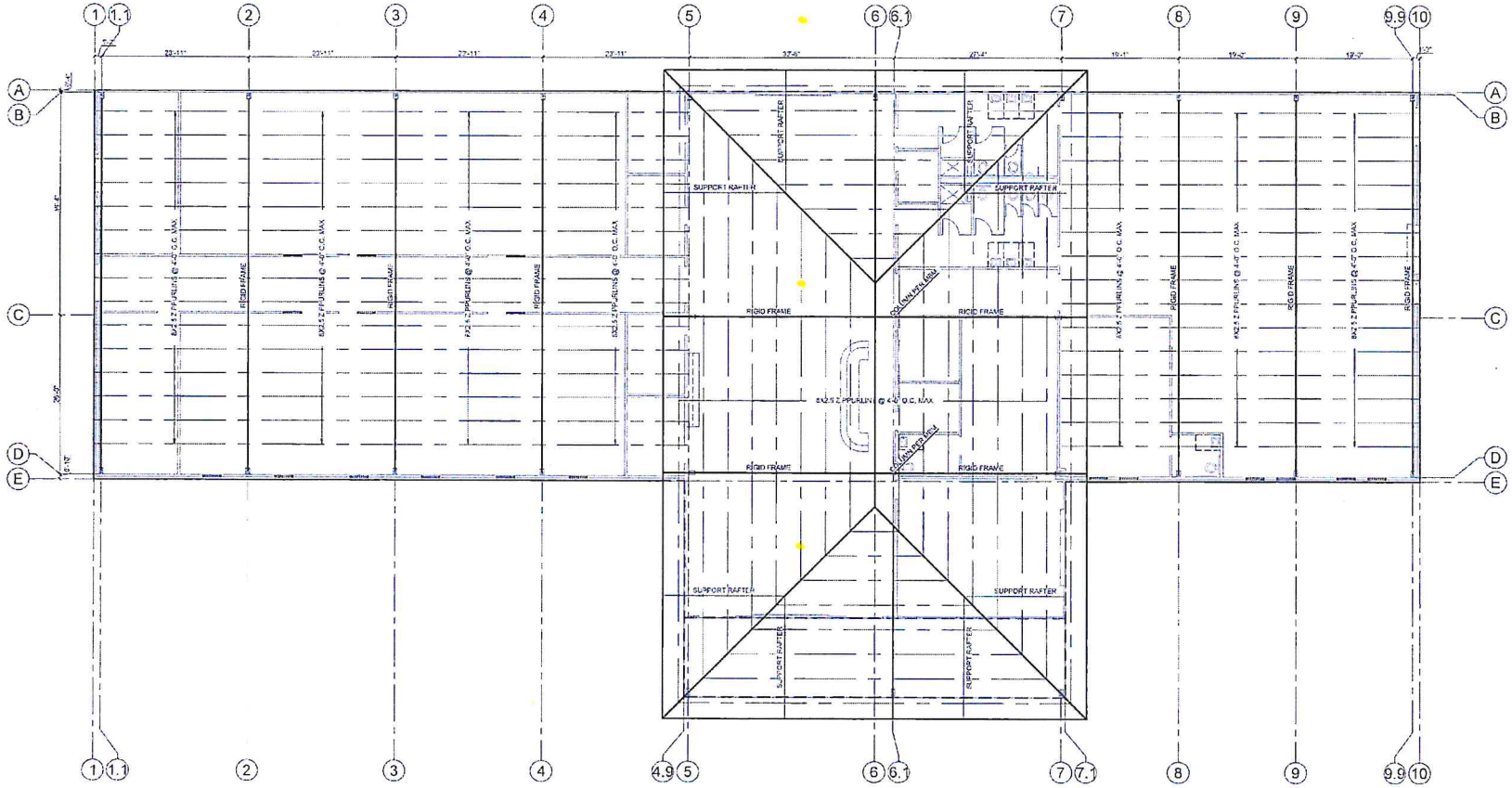
PROJECT: PROPOSED BUILDING ADDTION KINGSVILLE, TEXAS

REVISIONS		
NO.	DESCRIPTION	DATE

PROJECT NO. ST21-083
DATE 05/18/2021
DRAWN BY L.C.
SCALE AS NOTED
FOUNDATION PLAN
S1.2

FRAMING NOTES

1. FOR GENERAL NOTES SEE SHEETS S1.0 AND 1.1.
2. FOR TYPICAL DETAILS SEE SHEETS SD1 & SD2
3. SEE ARCHITECTURAL ROOF PLAN FOR ROOF HATCHES.
4. INTERIOR SUPPORT COLUMNS 4" DIAMETER x 1/4" PIPE UNO.
5. ALL COLUMNS AND MEMBERS CONNECTED AS PER MBM SPECIFICATIONS.
6. CONTRACTOR TO VERIFY ALL DIMENSIONS AND MEASUREMENTS PRIOR TO CONSTRUCTION. ANY VARIATIONS MUST BE BROUGHT TO ATTENTION OF THE ENGINEER PRIOR TO CONSTRUCTION.
7. FRAMING PLAN FOR BIDDING PURPOSES. ACTUAL DIMENSIONS MY VARY BASED ON MBM REQUIREMENTS.
8. INTERIOR AND EXTERIOR 4" METAL WALL STUDS @ 16" O.C.
9. ALL STEEL EXPOSED TO WEATHER SHALL BE HOT DIP GALVANIZED.



FRAMING PLAN

SCALE: 3/8" = 1'

ATLAS ENGINEERING CONSULTANTS
TBPE FIRM NO. 13907

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PROJECT: PROPOSED BUILDING ADDTION KINGSVILLE, TEXAS

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO: ST21-083

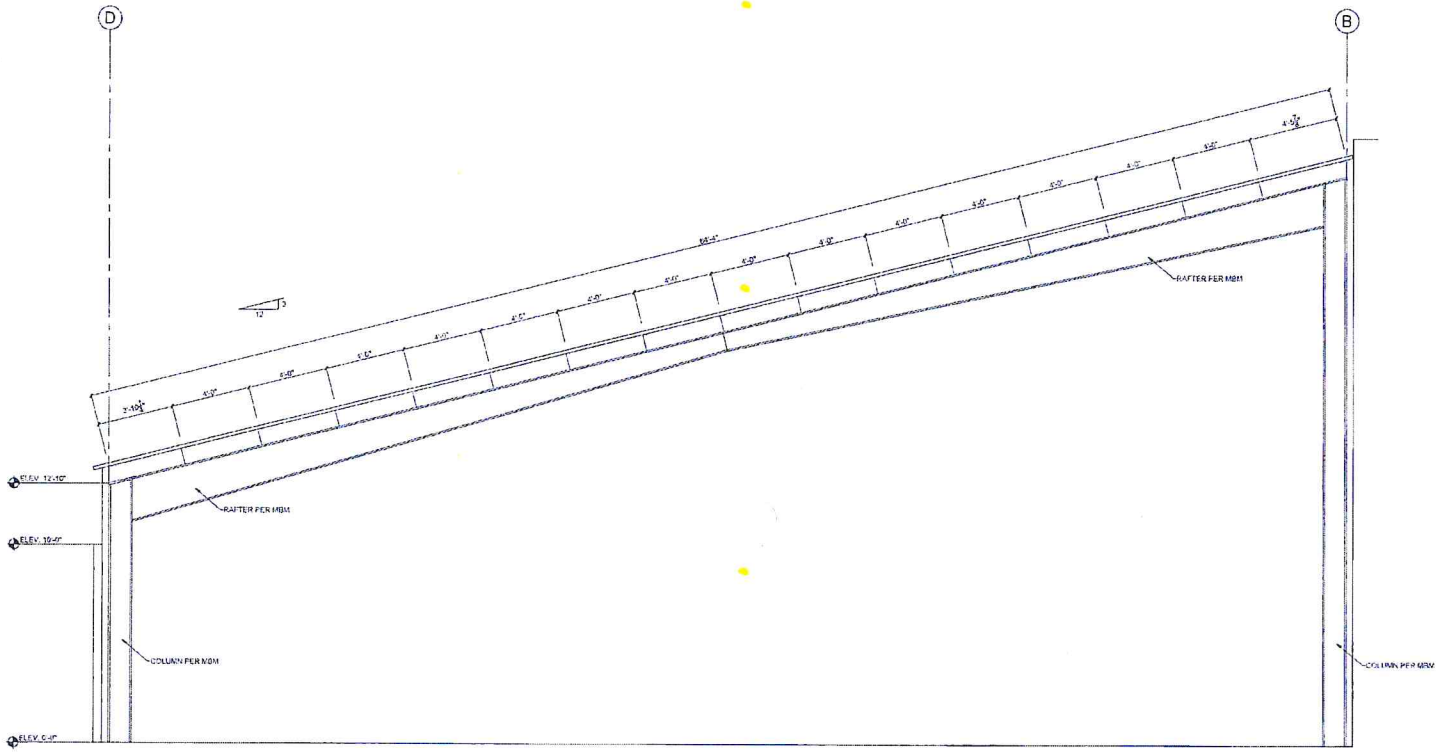
DATE: 05/18/2021

DRAWN: L.C.

SCALE: AS NOTED

FRAMING PLAN

S1.3



ENDWALL FRAMING

SCALE: 3/8" = 1'

NOTES:

1. REFER TO ARCHITECTURAL PLANS FOR ACTUAL HEIGHT REQUIREMENTS.

ATLAS ENGINEERING CONSULTANTS
TBPE FIRM NO. 13787
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KINGSVILLE, TX

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PROJECT:
PROPOSED BUILDING ADDTION
KINGSVILLE, TEXAS

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO. ST21-093

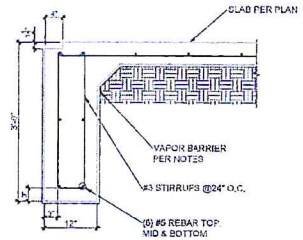
DATE 05/18/2021

DRAWN BY L.C.

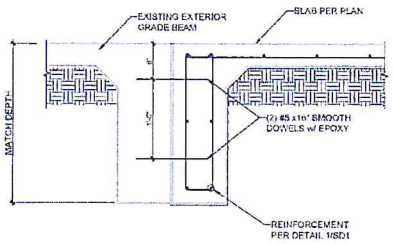
SCALE AS NOTED

ENDWALL

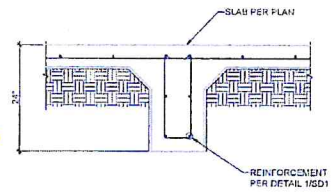
S1.4



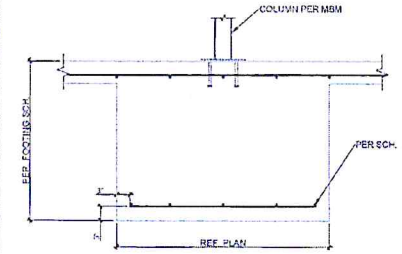
1. EXTERIOR GRADE BEAM



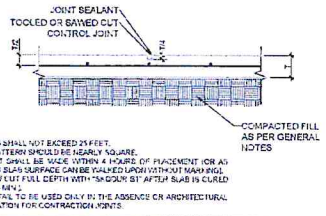
2. EXTERIOR GRADE BEAM



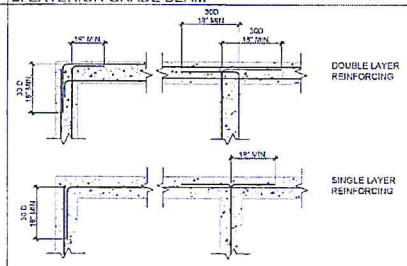
3. INTERIOR GRADE BEAM



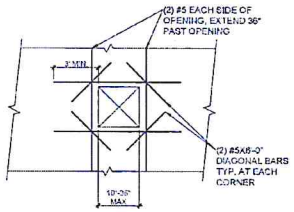
4. FOOTING AT COLUMN



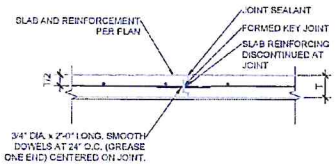
5. SLAB CONTROL JOINT



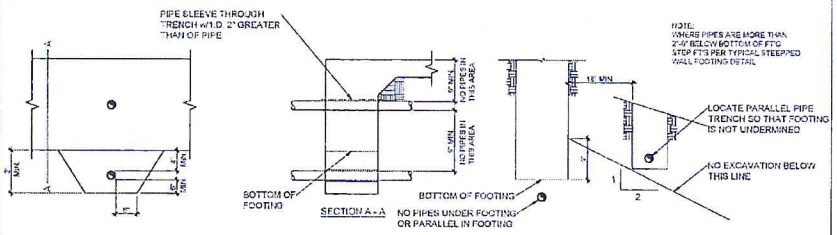
6. REINFORCEMENT AT CORNERS & INTERSECTIONS



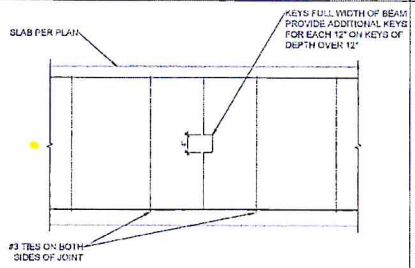
7. REINFORCEMENT AT PEDESTAL



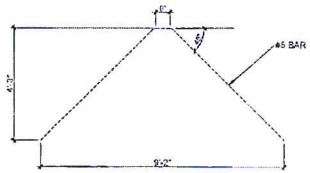
8. SLAB CONSTRUCTION JOINT



11. PIPING AT FOOTING



10. GRADE BEAM CONSTRUCTION JOINT.



10.1. HAIRPIN DETAIL

NOT USED

NOT USED

NOT USED

NOT USED

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PROJECT:
PROPOSED BUILDING ADDTION
KINGSVILLE, TEXAS

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO.	ST21-083
DATE	05/18/2021
DRAWN BY	L.C.
SCALE	AS NOTED
	FOUNDATION DETAILS
	SD1

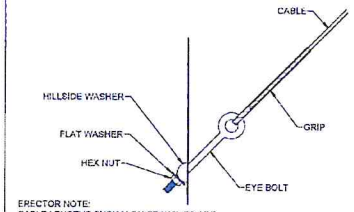
LINTEL SCHEDULE			
SIZE (SEE NOTE 4.)	CLEAR OPENING GREATER THAN	CLEAR OPENING UP TO	REMARKS
2L4x4x3/8	—	5'-0"	MIN. OF 8" BEARING AT EACH END
2L6x4x3/8	5'-0"	11'-0"	MIN. OF 8" BEARING AT EACH END
2L7x4x3/8	11'-0"	12'-0"	MIN. OF 8" BEARING AT EACH END
2L8x4x1/2	12'-0"	14'-0"	MIN. OF 8" BEARING AT EACH END

NOTES:
1. PROVIDE ONE MEMBER FOR EACH 4" (NORMAL) WIDTH OF MASONRY.
2. INSTALL ALL LOOSE LINTELS WITH LONG LEG VERT.
3. PROVIDE SHORING UNDER LINTELS OVER 6'-0" LONG UNTIL GROUT HAS SET 72 HOURS.
4. ANGLE SIZE IS MIN. COORDINATE REQUIRED HORIZ. LEG DIMENSION W/ARCH. DRAWING.

TYPICAL LINTEL DETAILS

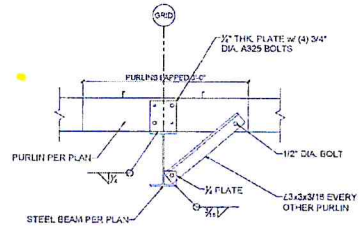
- FOR SUPPORT OF MASONRY WEIGHT ONLY.
- DEFLECTION AT L/600
- LINTELS SHALL BE GALVANIZED STEEL

11. STEEL LINTEL SCHEDULE

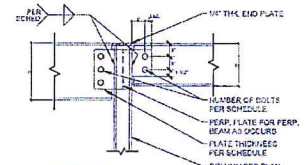


ERECTOR NOTE:
CABLE LENGTH IS SHOWN ON DRAWINGS ARE FROM END OF CABLE TO END OF CABLE. FIELD CUT AND ASSEMBLE BRACE AS REQUIRED

12. TENSION ROD DETAIL



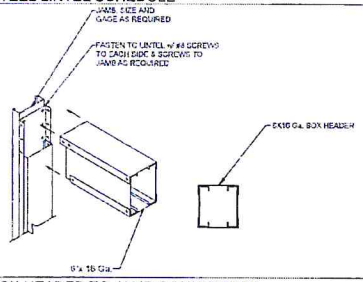
13. PURLIN TO RAFTER CONNECTION



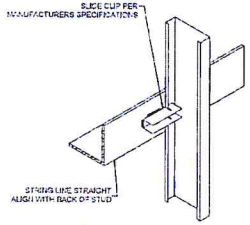
SIMPLE BEAM CONNECTION SCHEDULE			
BEAM DEPTH "d"	SHEAR STIFF PLATE	NO. & SIZE OF A325N BOLTS	WELD SIZE
8-12	1/4	(2) 5/8" DIA.	1/4"
12-14	1/4	(3) 3/4" DIA.	1/4"

NOTE:
TRANSVERSE BEAM SET BELOW TOC AT A DEPTH EQUAL TO PLATE/DPW PLATE PLUS 1/2".

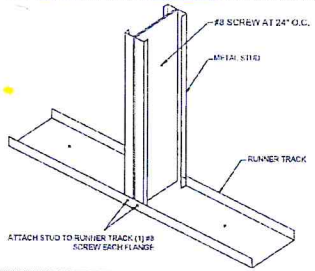
14. STEEL BEAM/CHANNEL TO STEEL COLUMN



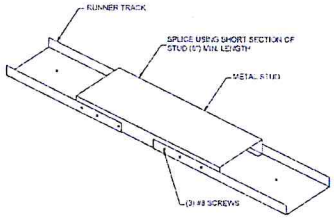
15. BOX HEADER TO JAMB CONNECTION



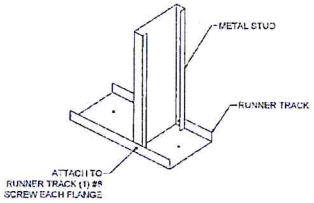
16. SIDE CLIP DETAIL



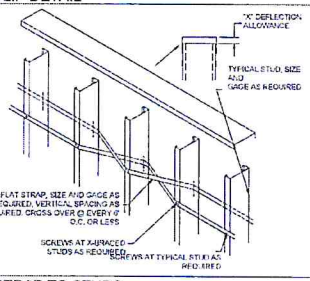
17. DOUBLE STUD TO RUNNER TRACK



18. RUNNER TRACK SPLICE



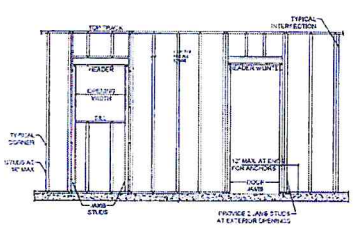
19. STUD ATTACHMENT TO RUNNER TRACK

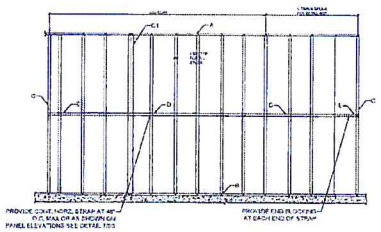


20. FLAT STRAP TO STUDS

NOT USED

NOT USED





NOTE:
1. MEMBER SIZES SHOWN IN THIS DETAIL ARE TYPICAL EXCEPT AS OTHER WISE SHOWN ON THE PLAN OR SPECIFIC PANEL ELEVATIONS.
2. ALL STUDS SHALL BE SPACED AT 16" O.C. EXCEPT AS SHOWN OTHERWISE AND AS NOTED BELOW.
3. LOAD BEARING STUDS SHALL BE SPACED SO AS TO FALL DIRECTLY UNDER ROOF TRUSSES/RAFTERS OR UNDER FLOOR JOISTS.

MARK	DESCRIPTION	SIZE
A	TOP TRACK	6" x 16 Ga.
B	BOTTOM TRACK	6" x 16 Ga.
C	STUD	6" x 16 Ga.
C1	DOUBLE STUD	(2) 6" x 16 Ga.
D	STRAP	2 x 16 Ga.
E	BLOCKING	6" x 20 Ga.

21. METAL STUD WALL

NOT USED

ATLAS ENGINEERING CONSULTANTS
THREE FINN NO. 17007
2400 Fitch Ave, Suite 1400
Fax 214-215-7
Tel 214-215-7
Email@atlaseng.com

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PROJECT: PROPOSED BUILDING ADDTION KINGSVILLE, TEXAS

REVISIONS		
NO.	DESCRIPTION	DATE

PROJECT NO. ST21-083
DATE 05/18/2021
DRAWN BY L.C.
SCALE AS NOTED
FRAMING DETAILS
SD2